

Design and Construction of Post-Tensioned Concrete Structures

FARHAD HUSSIAN

GIVING :

*To Mom and dad...To my family and my
clan...To my teachers..To my
colleagues...To burn candls to illuminate
for others..To each of the characters
taught me find dictate this modest Rajya
from Mawla...Almighty to find acceptanc
and success*

THE RESEARCH TARGET :

This research hope enable us to use this system in Kurdistan more than normal systems...

*This system consider economic (less steel , 50% less no. of crew , time ...etc
Also saving in labor and materials (mechanical , electrical , elevator , cladding)*

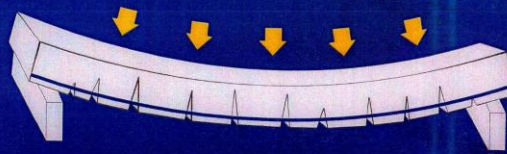
Session Outline

- **What is Prestressing?**
- **Overview of Post-Tensioning Systems**
- **Myths about Post-Tensioning**
- **Basic Design Concepts**
- **Advantages of Post-Tensioning**
- **Construction of Post-Tensioned Slabs**
- **Application of Post-Tensioning to Transfer Girders**

Prestressed Concrete

■ What is prestressing?

Prestressing is a method of reinforcing concrete. The concrete is prestressed to counteract the applied loads during the anticipated service life of the member.



Pre- and Post-Tensioned Concrete

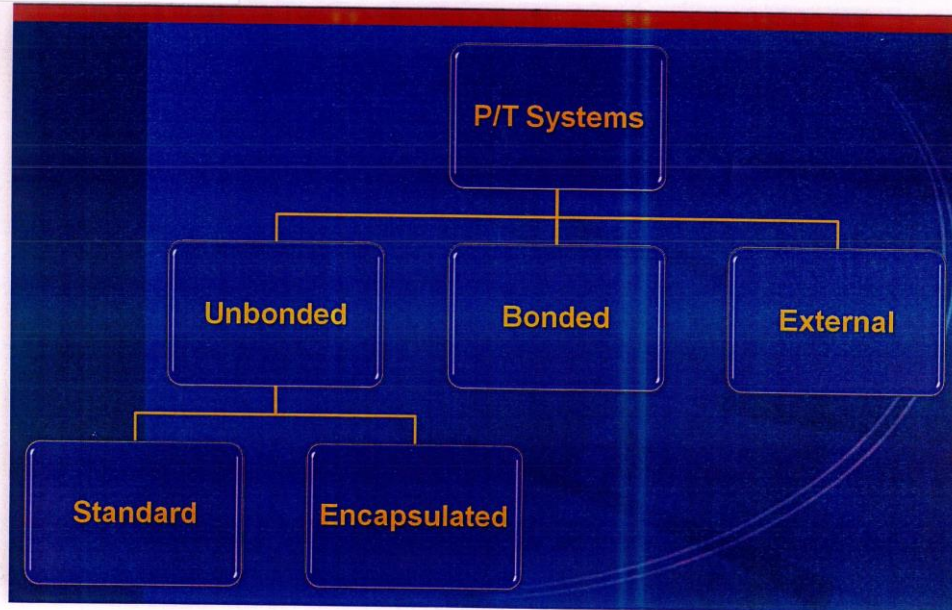
Pre-Tensioning:

Steel tendons are stressed prior to concrete placement, usually at a precast plant remote from the construction site

Post-Tensioning:

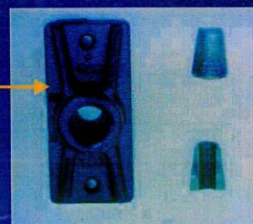
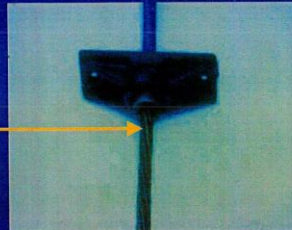
Steel tendons are stressed after the concrete has been placed and gained sufficient strength at the construction site

Post-Tensioning Systems

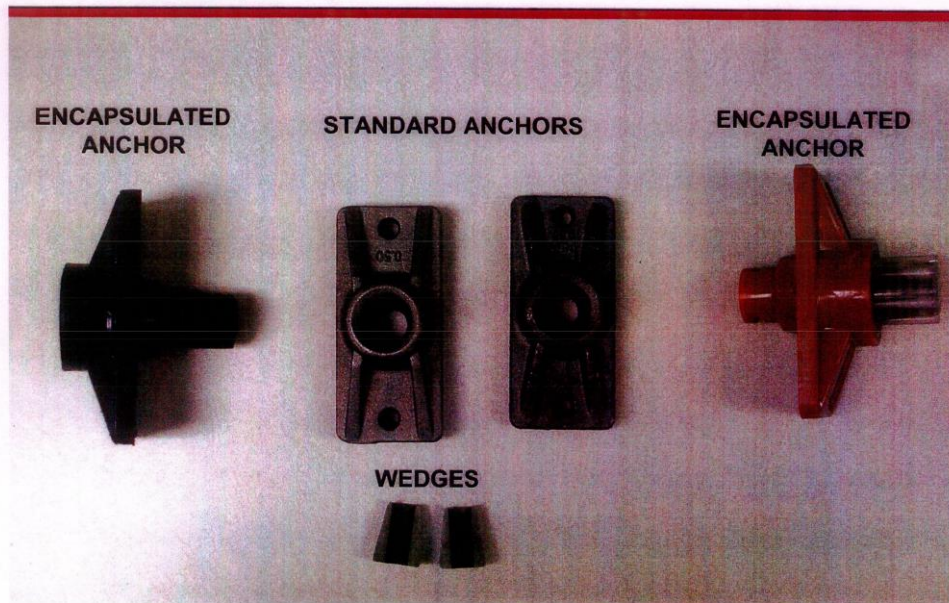


What is an Unbonded Tendon?

- Prestressing steel is prevented from bonding, and is free to move, relative to the surrounding concrete
- Consist of 7-wire strands
Most commonly used sizes are 0.5" dia strands
- The prestressing force is transferred to the concrete through the anchorages only



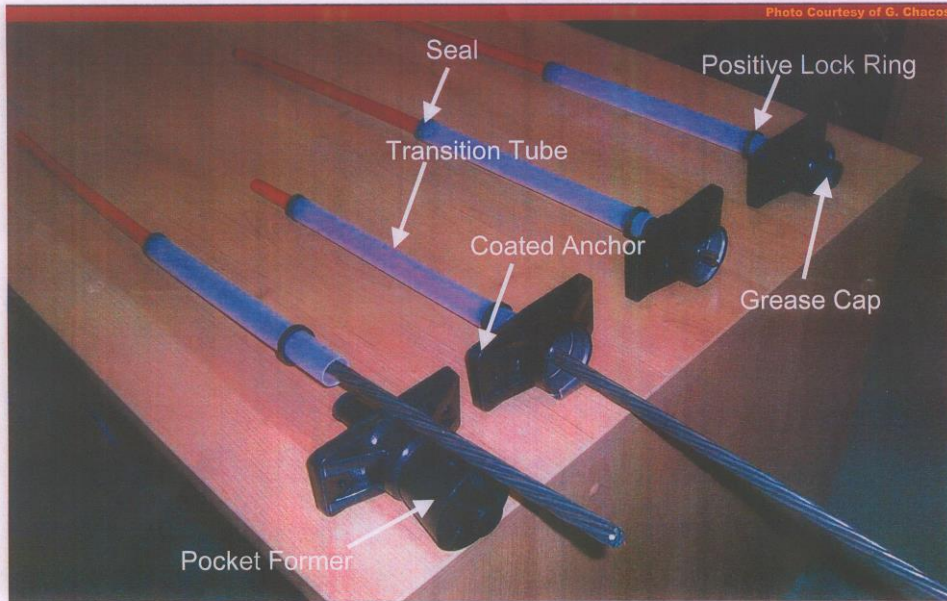
End Anchors



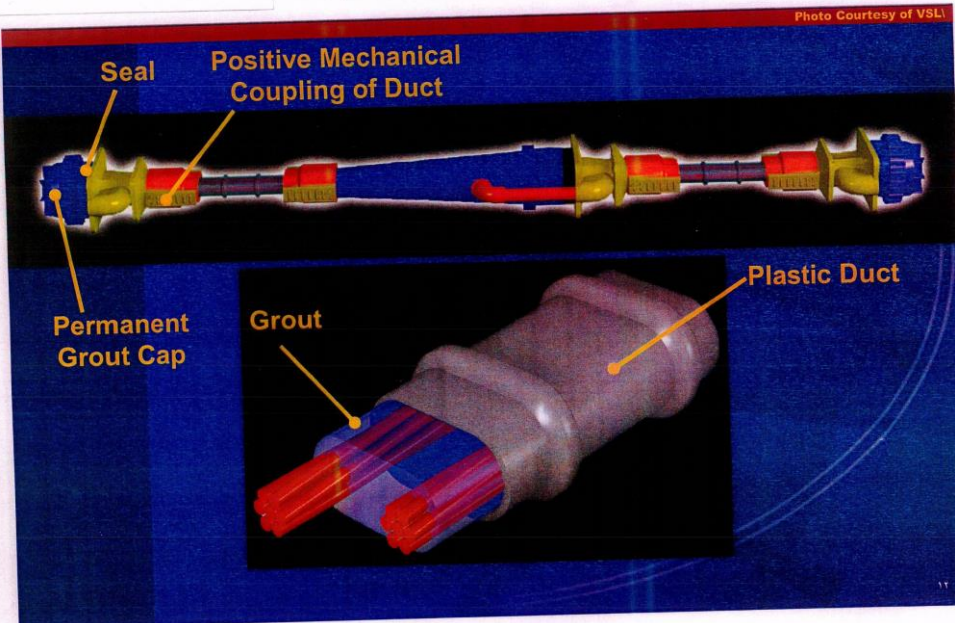
Encapsulated Tendons – When?

- Corrosive Environments
- Decks exposed and/or near the ocean
- Decks where de-icing salts (snow areas) will be used
- Slabs-on-ground where the soil has a high chloride content

Encapsulated Tendon Components



Bonded Tendons



Myths About P/T

- P/T Concrete is Crack Free
- P/T Concrete is Water Proof
- You Cannot Drill/Make Openings in P/T Slab
- If You Drill Into a Tendon, it will Fly Out of the Building
- It is Impossible to Upgrade/Repair a P/T Structure
- P/T Structures are Not Durable

Post-Tensioning Systems

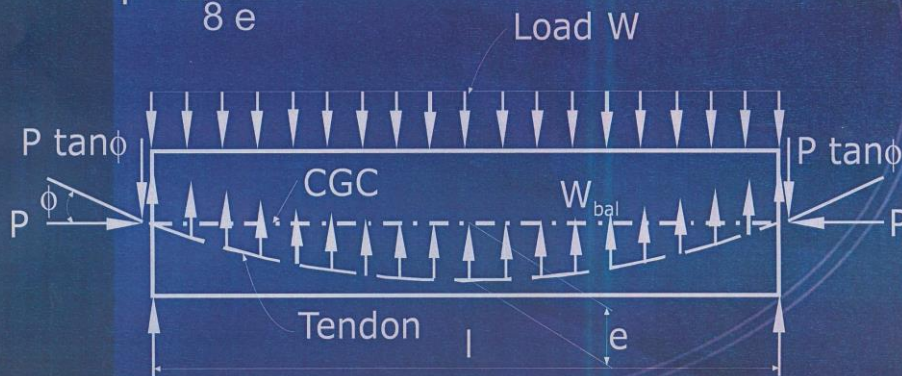
Unbonded vs. Bonded

- **Unbonded**
 - ❖ Economical
 - ❖ Greater layout flexibility
 - ❖ Force transmitted solely by the anchors
 - ❖ Total force limited by anchor spacing
 - ❖ Retrofit openings require more care
 - ❖ Replaceable
 - ❖ Simple stressing equipment
- **Bonded**
 - ❖ Can be more costly due to duct placement & grouting
 - ❖ Force transmitted by anchors and bond to concrete
 - ❖ Greater total force can be applied
 - ❖ Strain compatibility with concrete
 - ❖ Openings less difficult
 - ❖ Minimizes need for non-prestressed reinforcement
 - ❖ More complex stressing equipment required

Load Balancing

$M_1 = P \cdot e = \text{Primary Moment}$

$$P = \frac{W_{bal} l^2}{8 e}$$



Secondary Moment

$M_2 = \text{Secondary Moment}$

Developed in post-tensioned concrete members due to prestressing forces

Consequence of constraint by the supports to the free movement of the member

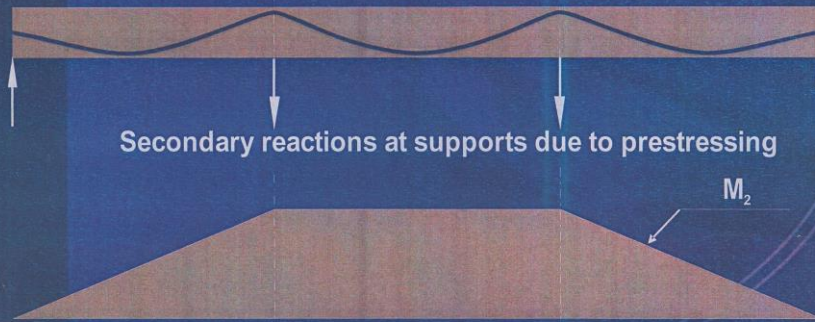
- Only develops in indeterminate members
- simply supported beams have zero secondary moments

Significant: must be accounted for in the design of prestressed concrete indeterminate structures

Secondary Moments – Cont'd

$$M_{bal} = M_1 + M_2 = Pe + M_{sec}$$

M_{bal} = Balanced moment by post-tensioning equivalent loads



Secondary Moments, M_2 , vary linearly between supports

Preliminary Design Span/Depth Ratio

| Floor System | Typical Span/Depth Ratio | |
|---|--------------------------|------------|
| | P/T Members | RC Members |
| One way slabs | 48 | 21-28 |
| Two-way slabs | 45 | 33 |
| Two-way slabs with drop panels (minimum drop panel $L/6$ each way) | 50 | 36 |

Cost Comparison Unbonded Tendons

| Slab Span (ft) | Slab Thick. (in.) | l/d ratio | Materials per Sq. ft ¹ | | | Cost per Sq. ft ² | | |
|------------------------|-------------------|-----------|-----------------------------------|-------------------|----------------|------------------------------|------------|--------|
| | | | Conc. (in.) | Reinf. Steel (lb) | P/T Steel (lb) | Conc. Cost | Steel Cost | Total |
| Non Prestressed | | | | | | | | |
| 30' X 30' | 10+4 | 36 | 10.3 | 4 | 0 | \$3.97 | \$1.80 | \$5.77 |
| Post-Tensioned | | | | | | | | |
| 30' X 30' | 8.5 | 42.4 | 8.6 | 0.8 | 0.9 | \$3.32 | \$1.44 | \$4.76 |
| 30' X 30' | 8 | 45 | 8.2 | 1.6 | 0.95 | \$3.16 | \$1.86 | \$5.02 |

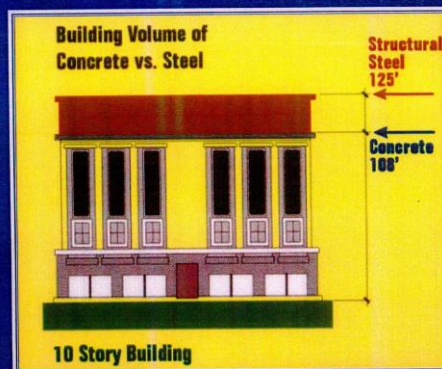
¹ Fintel, M., Ghosh, S.K., "Economics of Long-Span Concrete Slab Systems for Office Buildings - A Survey", Portland Cement Association, 1982

² Conc. Cost=\$125/Cubic Yd. Reinf. Steel Cost = \$0.45/lb P/T Steel Cost= \$1.20/lb

11

Comparison with Structural Steel Buildings

- Reduced structural depth for lower story heights, smaller column and foundation sizes
- Savings in labor and materials
 - ❖ Mechanical
 - ❖ Electrical
 - ❖ Elevator
 - ❖ Cladding



12

Post-Tensioned Concrete Advantages

- Long economical spans
- Effective use of high strength materials
- Permits wide flexibility in design variations
- Significantly reduces amount of non-prestressed bonded reinforcement over supports of continuous P/T structures

Post-Tensioned Concrete Advantages

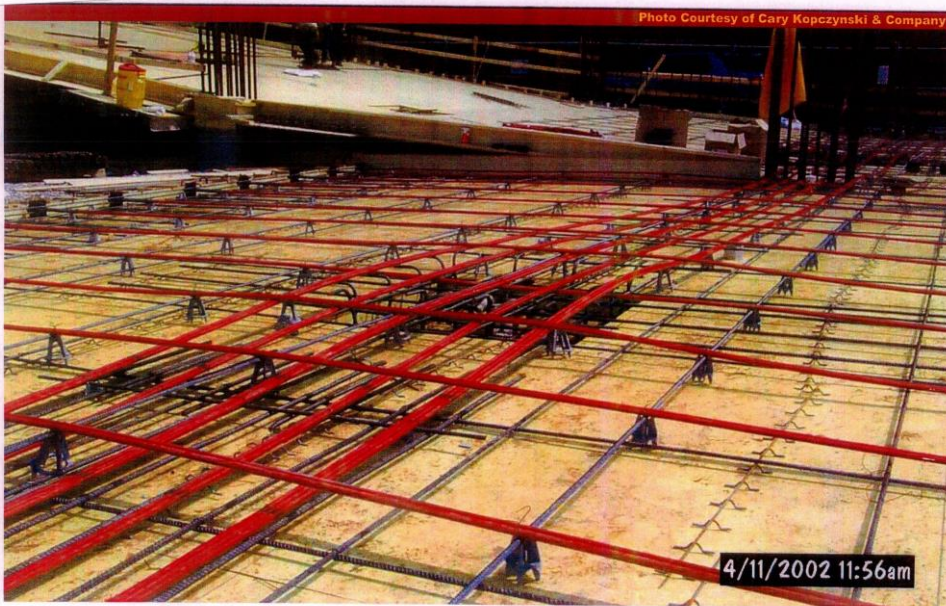
- Deflection and vibration control
- Crack control and water-tightness
- Noise control
- Lateral loads (rigid frame action)
- Seismic Loads (improved behavior of moment frames)

Construction Cycle

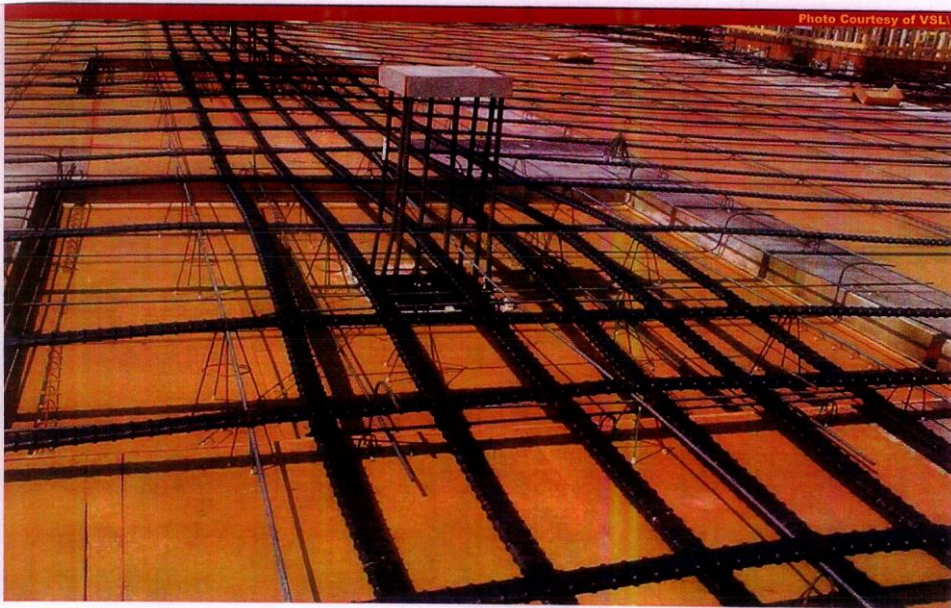
- Possible to achieve a 2-day cycle (may need overtime)
- Typical construction cycle
 - ❖ East coast 3 day (Low Seismic Areas)
 - ❖ West coast 3-4 day (Higher Seismic Areas)
- Typically need about 50% of crew for post-tensioned buildings
- Re-shoring is not required once the slab has been post-tensioned

12

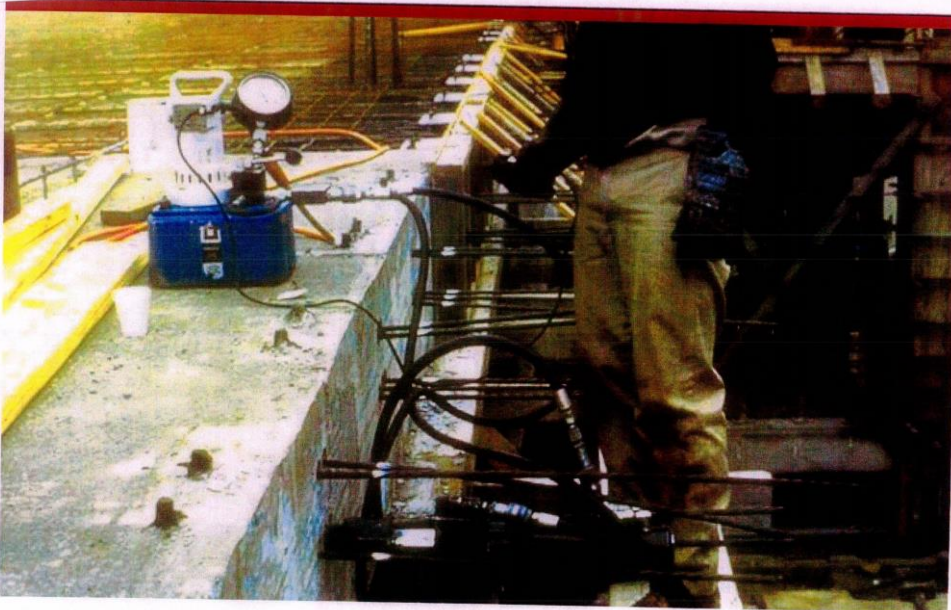
Tendon Installation



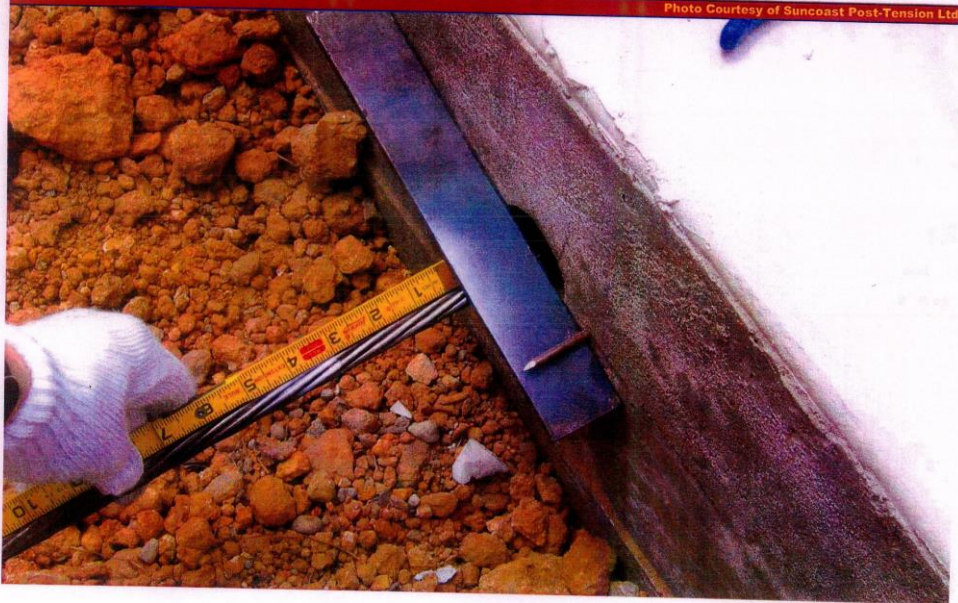
Two-Way Slab with Bonded P/T



Stress Tendons



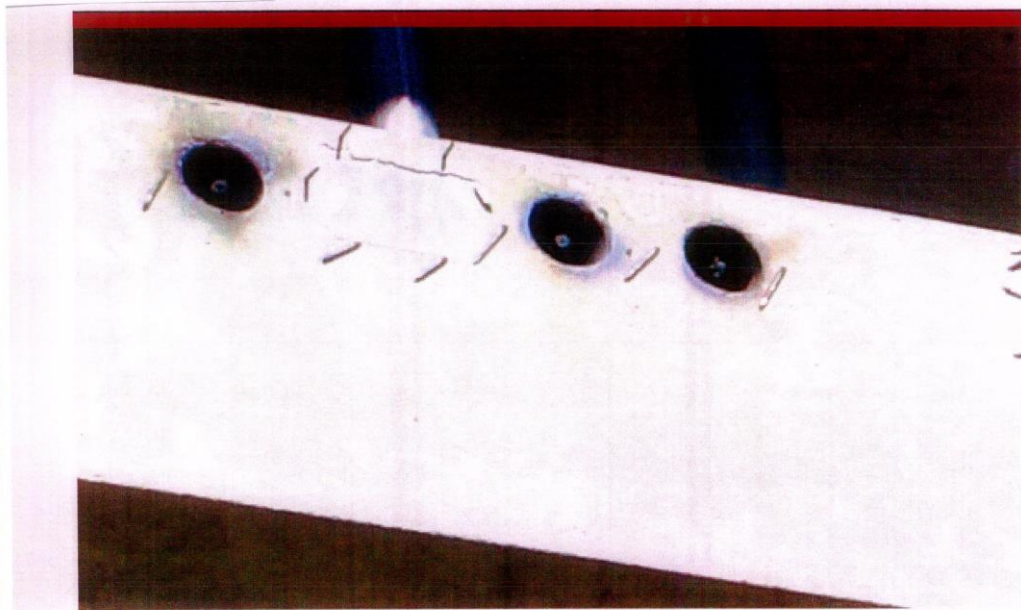
Measure Elongation



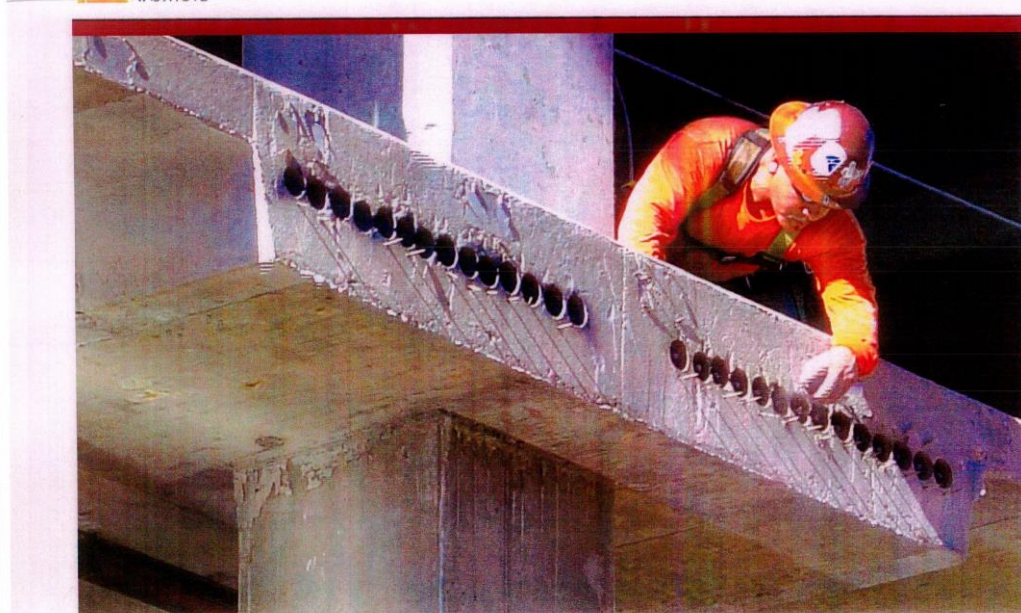
Elongations

- Measured elongation needs to be within 7% of calculated value
- Per code, for tendons outside the 7%, the EOR shall ascertain and correct the “problem”. What to do? De-Tension / Re-Tension?
- Elongation records shall be sent to the reviewing engineer by the end of the next working day after stressing
- Elongations shall be approved or rejected within three working days after stressing

Cut Tendon Tails



Clean Pockets



Patch End Pockets



End Installation for Bonded Tendons

Photo Courtesy of VSL

Attachment of Permanent Grout Cap



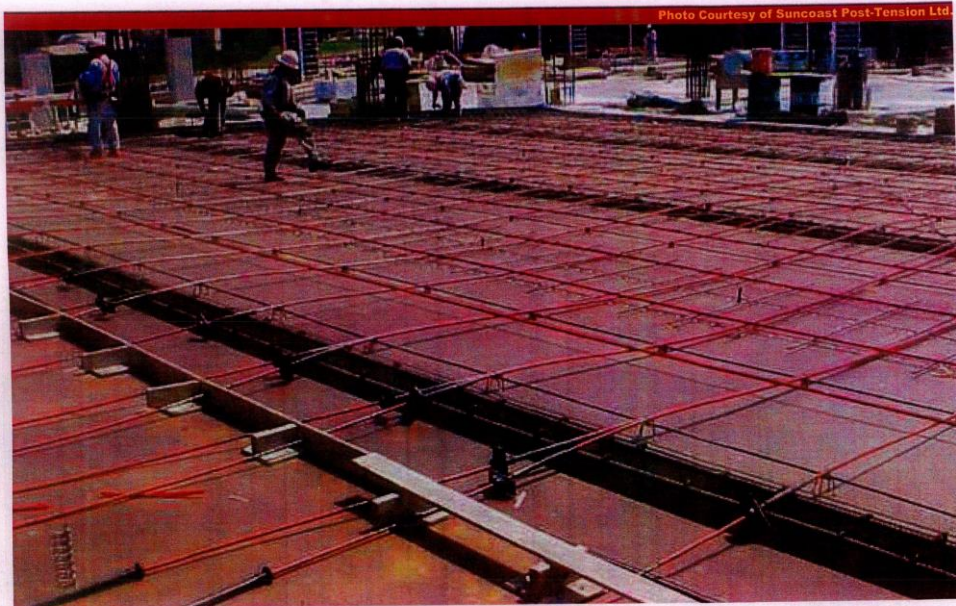
One-way Slabs and Beams

- Typically used in long span parking structures
- Slab spans typically between 18 to 30 feet
- Beams can clear span up to 65' at a 3'-0" system depth
- Maximum tensile stress is $12\sqrt{f'c}$

Post-Tensioned Parking Garage



One-Way Slab



Two-Way Slabs

- Typically used in Residential and Office Buildings
- Slab spans between 20 to 40 feet
 - ❖ Flat Plate 20 to 30 feet
 - ❖ Slabs with Drop Caps 25 to 35 feet
 - ❖ Flat Slab 30 to 40 feet
- Maximum tensile stress is limited to $6\sqrt{f'_c}$

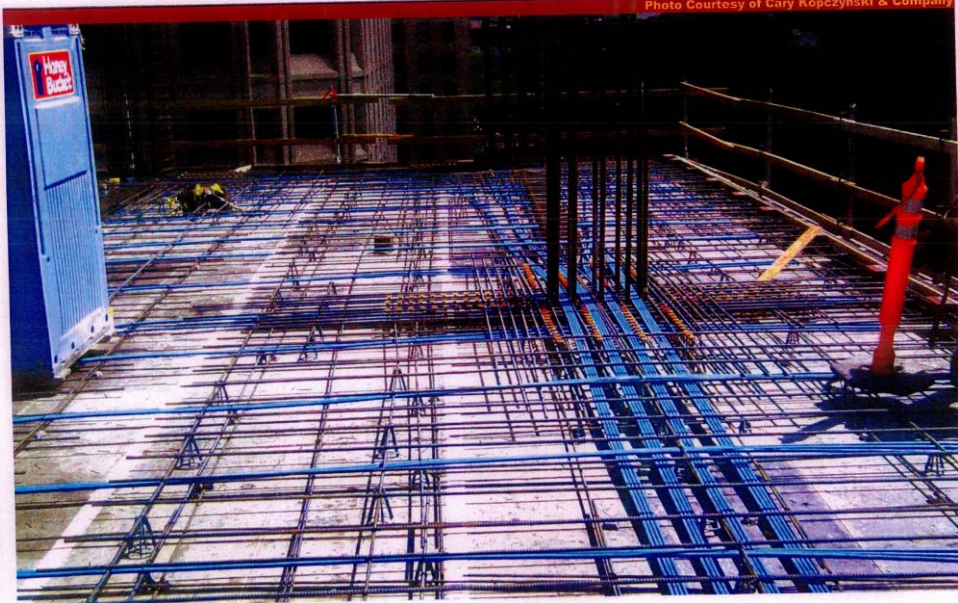
Two-Way Slabs

Photo Courtesy of Suncoast Post-Tension Ltd.

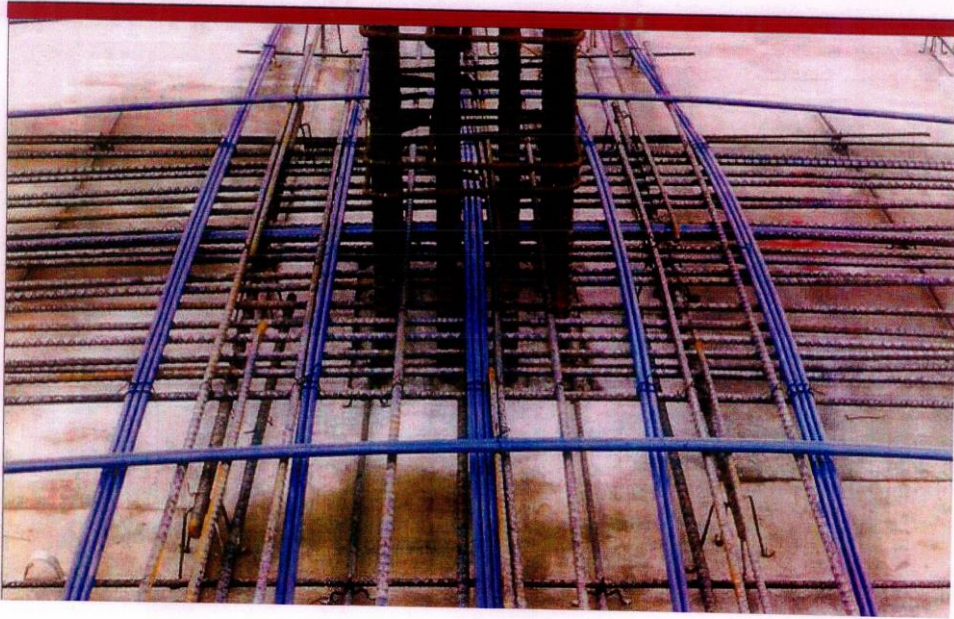


Two-Way Slab

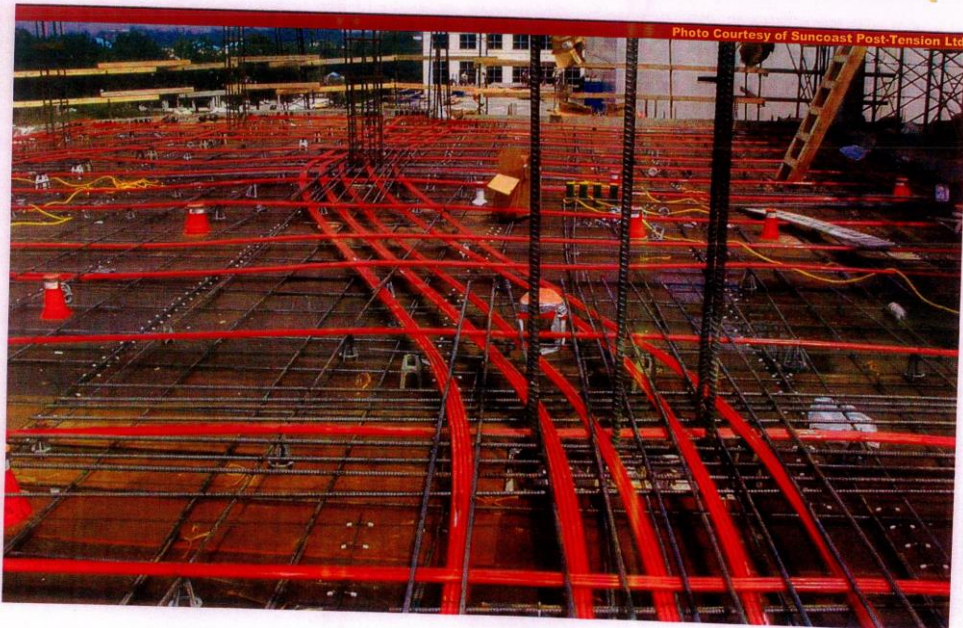
Photo Courtesy of Cary Koczynski & Company

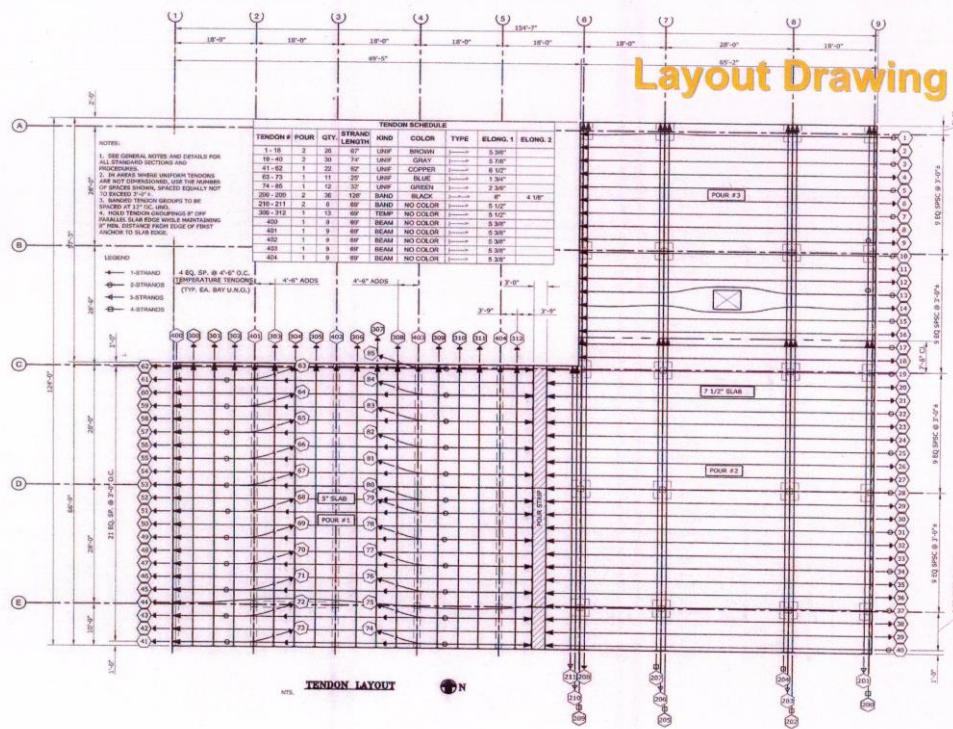
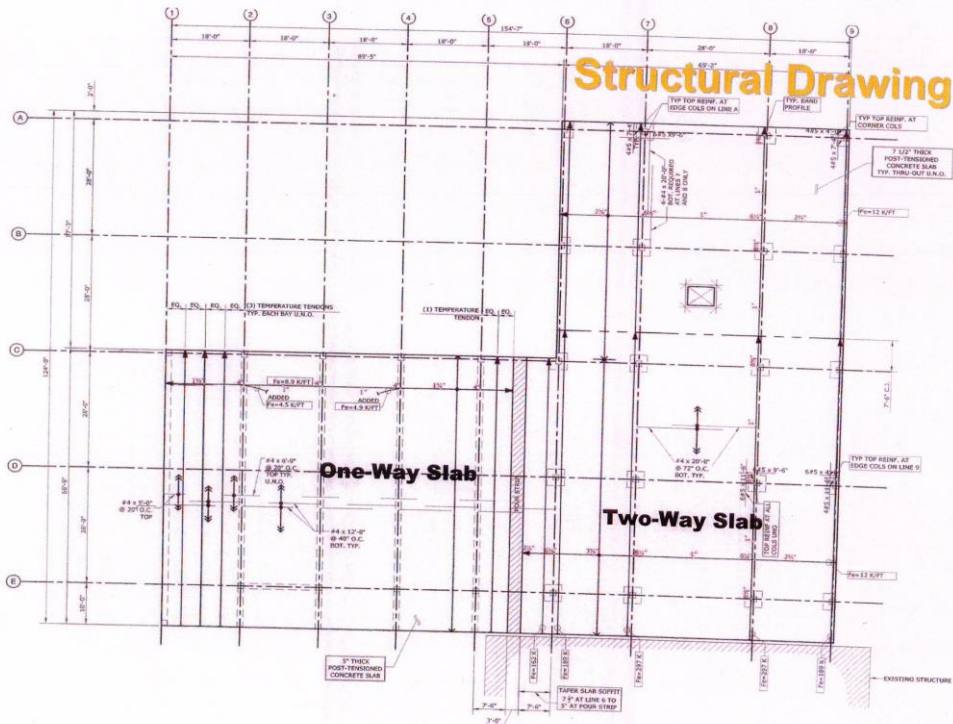


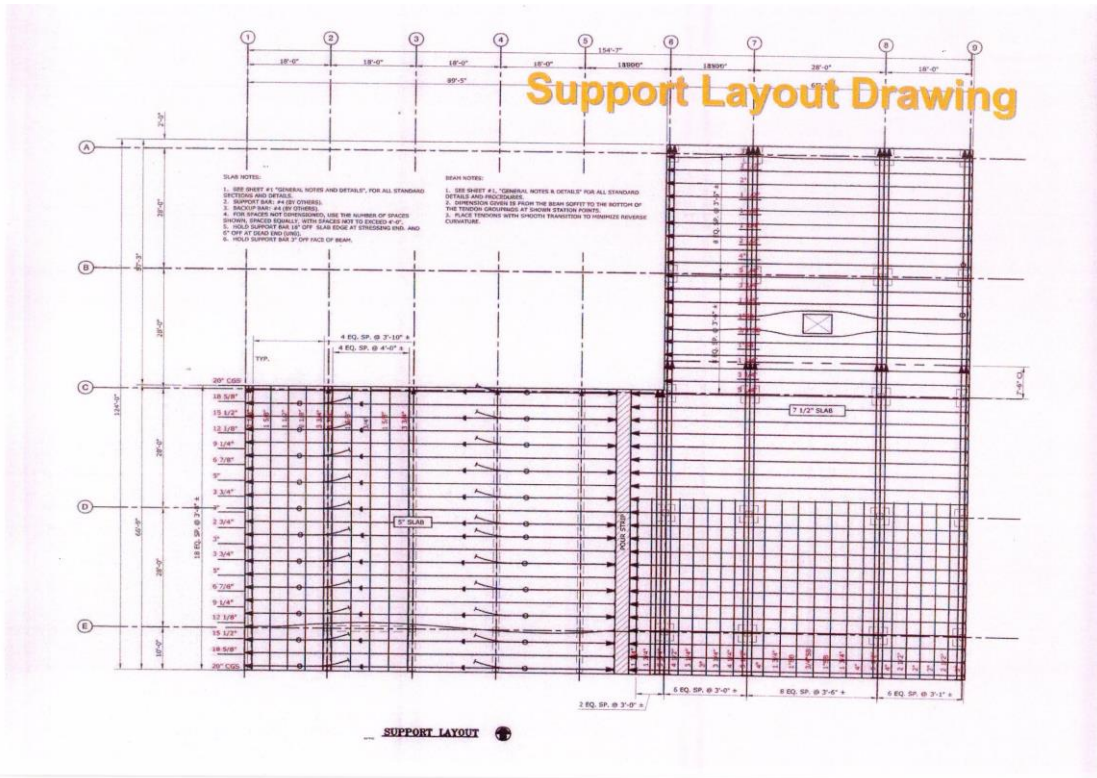
Tendon Layout in 2-Way Slabs



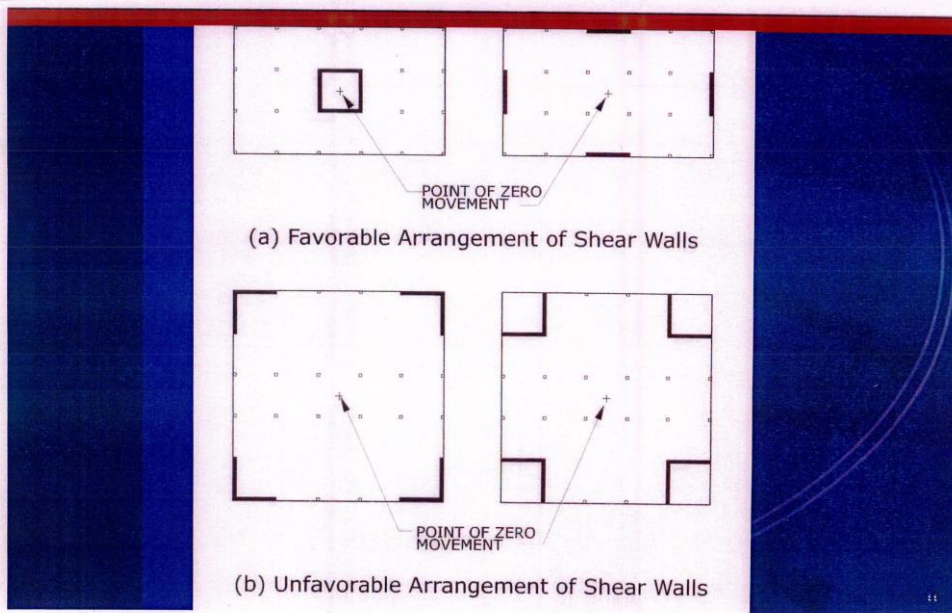
Tendon Sweep







Arrangement of Shear Walls



Floor Shortening and Restraint Cracking

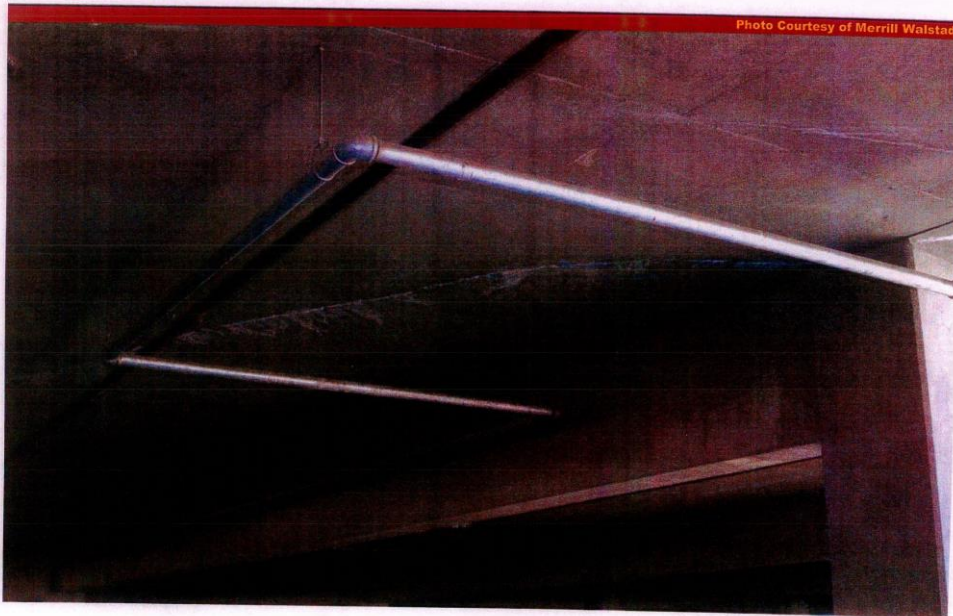
- Sources of Cracking
 - ❖ Short floor-to-floor height
 - ❖ Short stiff columns
 - ❖ Stiff lateral load resisting elements
- Factors Contributing to Floor Shortening
 - ❖ Elastic shortening due to precompression
 - ❖ Creep shortening due to precompression
 - ❖ Shrinkage
 - ❖ Temperature drop
- Show Joint and Separation Details on Structural Drawings
- Inspect Separation Details During Construction

10

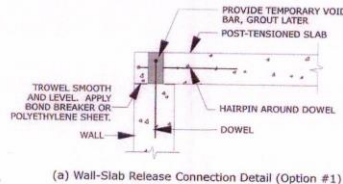
Restraint Cracking



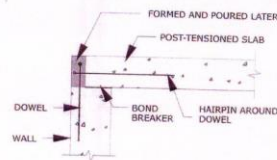
Restraint Cracks



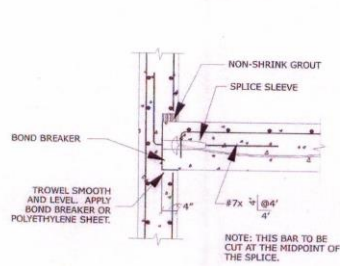
Movement Joints



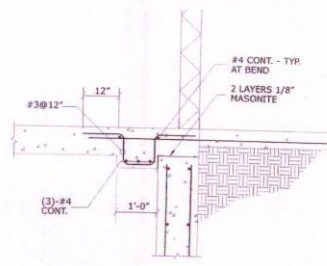
(a) Wall-Slab Release Connection Detail (Option #1)



(b) Wall-Slab Release Connection Detail (Option #2)

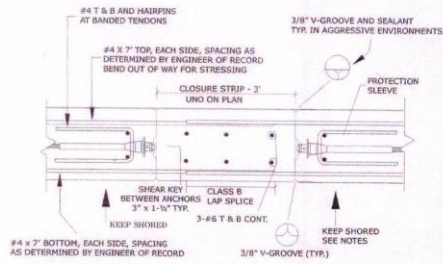


(c) Connection of Post-Tensioned Slab To Wall With Splice Sleeves



(d) Connection of Post-Tensioned Slab to Basement Wall

Closure Strip



- NOTES:
1. RETAIN SHORING UNTIL CONCRETE IN CLOSURE STRIP REACHES 75 PERCENT OF SPECIFIED SLAB CONCRETE STRENGTH.
 2. CLOSURE STRIP TO BE FILLED WITH NON-SHRINK CONCRETE.
 3. ROUGHEN AND CLEAN JOINTS. WET PRIOR TO PLACING CONCRETE.
 4. ELIMINATE ACCIDENTAL MISALIGNMENT BETWEEN EDGE OF SLABS THAT ARE TO BE JOINED WITH A CLOSURE STRIP. USE MECHANICAL METHODS SUCH AS JACKING IF NECESSARY.
 5. PROVIDE WATERPROOFING MEMBRANE IF REQUIRED FOR WATER-TIGHTNESS.
 6. IF CLOSURE STRIP WILL BE IN AN AGGRESSIVE ENVIRONMENT, SEAL ANCHORAGE POCKETS PER THE PROJECT SPECIFICATIONS AND CAULK JOINT ALL THE WAY AROUND WITH FLEXIBLE SEALANT.
 7. CONTRACTOR SHALL NOTE THAT THERE ARE SPECIAL SHORING CONDITIONS ON EITHER SIDE OF THE CLOSURE STRIP, ESPECIALLY FOR MULTI-STORY STRUCTURES.

- Slab is temporarily allowed to cure in smaller segments
- Can locate the lateral system in the middle of the individual pour
- Allows for internal stressing – may be critical on subterranean projects
- Typically remain open for 30 to 60 days. Deck will still move for many months/years

Transfer Girders



Cantilevers

Transferred Columns

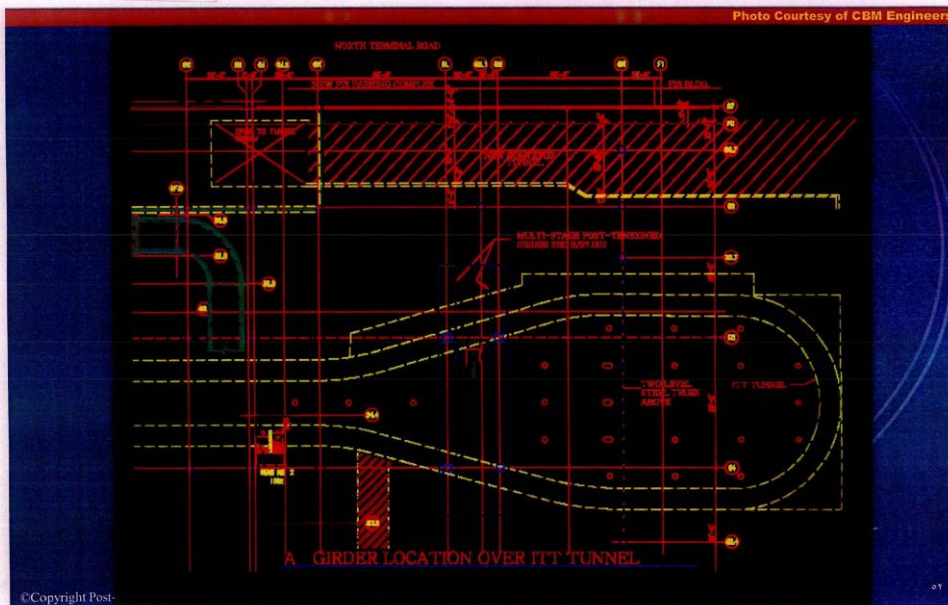
Transfer Girders



- Harped profile may be more efficient to resist concentrated loads
- P/T forces can balance the dead loads
- Stage stressing to avoid overstressing the beams
- Multi-strand tendons when large forces are required

Case Study

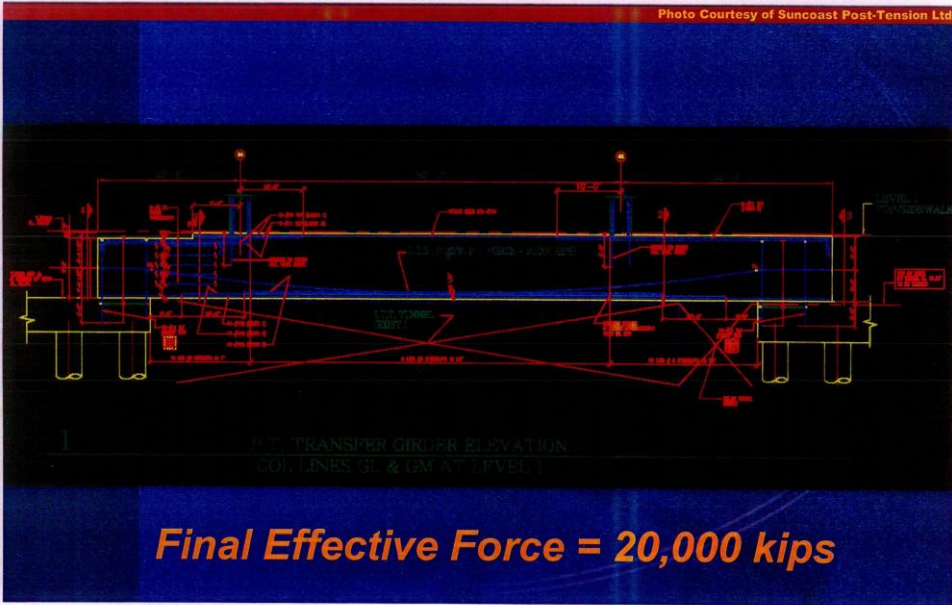
Transfer Girder over Tunnel



P/T Transfer Girder

Elevation at Level 1

Photo Courtesy of Suncoast Post-Tension Ltd.



Original Design

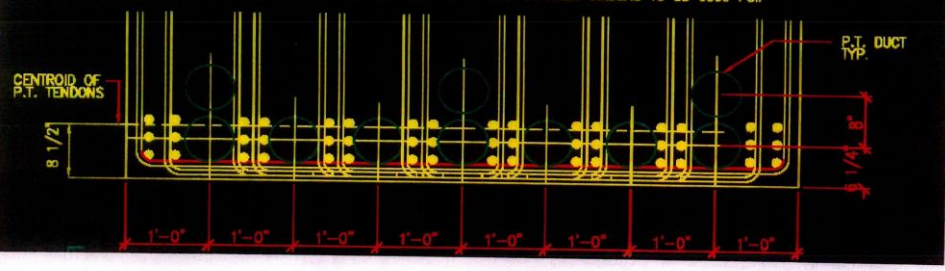
Bonded Tendon

Photo Courtesy of Suncoast Post-Tension Ltd.

STRESSING STAGES

- STAGE 1 - EFF. P.T. FORCE = 6000 KIPS , WHEN P.T. GIRDERS HAVE REACHED 4500 PSI STRENGTH
- STAGE 2 - EFF. P.T. FORCE = 10000 KIPS , AFTER LEVEL 2 FLOOR HAS REACHED MIN. 3000 PSI STRENGTH AND P.T. TENDONS ARE STRESSED.
- STAGE 3 - EFF. P.T. FORCE = 14000 KIPS , AFTER LEVEL 4 FLOOR HAS REACHED MIN. 3000 PSI STRENGTH AND P.T. TENDONS ARE STRESSED.
- STAGE 4 - EFF. P.T. FORCE = 18000 KIPS , AFTER LEVEL 6 FLOOR HAS REACHED MIN. 3000 PSI STRENGTH AND P.T. TENDONS ARE STRESSED.
- STAGE 5 - EFF. P.T. FORCE = 20000 KIPS , AFTER LEVEL 8 FLOOR HAS REACHED MIN. 3000 PSI STRENGTH AND P.T. TENDONS ARE STRESSED.

- 6. STRESSING WILL BE DONE IN A SYMMETRICAL WAY ABOUT VERTICAL AXIS FOR EACH STAGE.
- 7. TENDONS WILL BE GROUTED AFTER ALL TENDONS IN GIRDER HAVE BEEN FULLY STRESSED & INSPECTED.
- 8. 28 DAY CONCRETE STRENGTH IN TRANSFER GIRDERS & SLAB BETWEEN GIRDERS TO BE 6000 PSI.



Revised Design

Unbonded Post-Tensioned

Photo Courtesy of Suncoast Post-Tension Ltd.



Construction

Photo Courtesy of Suncoast Post-Tension Ltd.



End Anchorage Layout



Tendon Layout



Reasons to Consider P/T

- P/T slab is typically 30% thinner
- Long-term creep problems are virtually eliminated by load balancing
- Moment of inertia approaches I_{gross}
- The slab can be stressed and the forms removed in 2-3 days
- The 21 to 28 day shoring time for rebar concrete does not apply to P/T
- Flexibility in Column Layout
- Large Cantilevers

Thank You!

